# Traffic Impact STUDY 

# NEW CENTURY FILM Neversink Drive at NYS Route 209 <br> Town of Deerpark <br> Orange County, New York 

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The purpose of this Traffic Impact Study is to identify potential adverse traffic issues that may result due to the development of various improvements and expansions of an existing film studio on property located on Neversink Drive and Route 209 in the Town of Deerpark, New York. The proposed project would include hotel rooms, a multi-purpose building and new restaurants and film studio. The entire site will maintain its existing access from driveways on Neversink Drive. The access to Route 209, which is under the jurisdiction of the New York State Department of Transportation (DOT), will be for emergency use only. Neversink Drive is a designated County Road - CR-80. The project build-out is estimated to be about three years, i.e., completed and occupied in 2025.
The site and its environs are shown in the following map and plan - a Google Earth aerial map showing the approximate boundaries of the site and a plan from the application plan set prepared by Fellenzer Engineering LLP:

## SITE LOCATION MAP



Source: Google Earth

## SITE PLAN



Source: Fellenzer Engineering LLP

## Existing Conditions

The following is a description of existing travel conditions near the site of the New Century Film site:

## Roadways

Route 209 is a two-lane generally northeast/southwest major arterial running through the Town, lying just northwest of the project site. Route 209 directly serves both residential and commercial uses. Some residential uses have direct access to Route 209 and in the vicinity of the site there are several intersecting side roads - including Peenpack Trail to the north of the site and Hangar Drive to the south in Deerpark.

There is no on-street parking on Route 209 and the pavement is in good condition. There are two different posted speed limits -45 mph at the project site starting northeast of the Huguenot property and continuing northeastward, and 40 mph to the southwest through Deerpark and past Hanger Drive. At the intersections of Route 209 with Neversink and Hangar Drives, traffic on the side roads is controlled by a Stop sign while Route 209 traffic is
free flowing. Travel lanes are 12 feet in width and there are 5 -foot wide shoulders on both sides.

Neversink Drive is a two-lane County Road (CR-80) that intersects Route 209 at the project site. Its alignment at Route 209 is generally east/west, however, farther south, Neversink Drive curves to a more north/south alignment. It serves primarily residential land uses. The pavement is in good condition and the posted speed limit is 45 mph .

At one time, the speed limit on Neversink Drive was 55 mph and was the subject of a 2015 resolution from the Town Council to the Orange County DPW to review speeds and safety conditions that residents and the Town felt were excessive. That study led to the reduction of the limit to the now posted 45 mph and a number of curve warning signs accompanied by lower advisory speed limits.

Parking is not prohibited on Neversink Drive; however, street parking is rare as virtually all abutting properties have driveways and off-street parking spaces. The roadway width is 22 to 24 feet near the approach to Route 209.

Peenpack Trail is a two-lane local road that intersects Route 209 to the north of the project site. Its alignment at Route 209 is generally east/west. It serves primarily residential land uses. The pavement is in good condition and the posted speed limit is 45 mph .

Parking is not prohibited on Peenpack Trail; however, street parking is rare as virtually all abutting properties have driveways and off-street parking spaces. The roadway width is 22 to 24 feet near the approach to Route 209.

Hangar Drive is a two-lane local road that intersects Route 209 to the south of the project site. Its alignment at Route 209 is generally east/west. It provides partial access to a large residential trailer park property known as the Huguenot. The pavement is in good condition and the posted speed limit is 30 mph at Route 209 and lowers to 25 mph within the Huguenot.

Parking is not prohibited on Hangar Drive; however, street parking is rare as virtually all abutting properties have driveways and off-street parking spaces. The roadway width is $\pm 28$ feet near the approach to Route 209.

## Traffic Volumes

As proposed, the development will include hotel, restaurant and film studio uses. To evaluate the potential "worst-case" impacts of this type of development, manual turning movement counts were conducted at four key intersections on typical weekday (i.e., non-holiday/recess, school in session) mornings and evenings. The times were 7:00 to 9:00 AM and 4:00 to 6:00 PM on Thursday, October 27, 2022. The counted intersections were as follows:

1. Route 209 and Peenpack Trail
2. Route 209 and Neversink Drive
3. Route 209 and Hanger Drive
4. Neversink Drive and New Century Film Driveway

At all study intersections the minor side-streets are controlled by Stop signs. The AM and PM peak hour volumes (i.e., the highest 60 -minute periods) were reduced from the collected 2 hour volumes, which are contained in Appendix A. The peak hour volumes are shown graphically in Figures 1 and 2 in Appendix B for the AM and PM peak hours, respectively.

## Level of Service Analysis - Existing Conditions

The 2016 Highway Capacity Manual - $6^{\text {th }}$ Edition (HCM), published by the Transportation Research Board, defines Level of Service (LOS) for signalized and unsignalized intersections as a function of the average vehicle control delay. LOS may be calculated per movement or per approach for any intersection configuration, but LOS for the intersection as a whole is only defined for signalized and all-way stop configurations. In this analysis, the study locations are both two-way Stop controlled intersections with " T " shaped configurations.

Delay is defined in the HCM 2016 as "the additional travel time experienced by a driver, passenger, bicyclist, or pedestrian beyond that which is required to travel at the desired speed."

For unsignalized intersections (i.e., Stop sign controlled), the major road has free through movements while movements from the minor road are controlled by a stop sign. The movements that are subject to control delays are rated on a scale of "A" to "F," with LOS "A" exhibiting very short delays -10 seconds or less on average - and LOS " F " exhibiting much longer delays - 50 seconds or more per vehicle on average. The relationship of LOS to delay times is shown in the following table:

TABLE 1: LEVEL OF SERVICE VS. DELAY TIMES STOP/YIELD SIGN CONTROLLED INTERSECTIONS

| LOS <br> (Unsignalized <br> Intersections) | Average Control Delay <br> (sec/vehicle) |
| :---: | :---: |
| A | $\leq 10 \mathrm{sec}$ |
| B | $>10-15 \mathrm{sec}$ |
| C | $>15-25 \mathrm{sec}$ |
| D | $>25-35 \mathrm{sec}$ |
| E | $>35-50 \mathrm{sec}$ |
| F | $>50 \mathrm{sec}$ |

In the two-way Stop controlled Level of Service analyses, the through movements on the major road and right turns from the major road are assumed to have no delay. LOS for those movements is not an integral part of the analysis, because LOS is determined by control delay, and for these "free" movements, the control delay is zero.

Movements that are subject to small to moderate control delays include left turns from the major road, through movements on the minor road and right turns from the minor road. Movements that are most affected by control delay include left turns from the minor road.

Generally accepted software (Synchro) was used to compute control delays and Levels of Service. Synchro uses the methodologies published in the Highway Capacity Manual and requires input from the user specific to the intersections being studied. Among other items, that input information includes the following:

1. Traffic Volumes - from the manual counts noted above.
2. Speeds - from field observations of posted limits and advisories as noted above.
3. Lane Configuration and Width - from field measurements.
4. Traffic Control - from field observations that included Stop/Yield control or timings and phasing if signal controlled.
5. Peak Hour Factor - from the manual counts noted above.
6. Vehicle Mix/Classification - from NYSDOT counts, which indicate $7 \%$ heavy trucks/buses on Route 209, 5\% heavy vehicles on Neversink Drive and the default 2\% heavy vehicles on the local streets.
7. Buses - from field observations indicating no route buses with stops any study street.
8. Pedestrians/Bicycles - from field observations indicating few if any pedestrians and bicycles.

The Levels of Service and corresponding control delays for the study locations are summarized in the following Table for the AM and PM peak hours. The detailed LOS summary reports are contained in Appendix C.

TABLE 2: LEVEL OF SERVICE SUMMARY - EXISTING CONDITIONS

| INTERSECTION | MVMT. | EXISTING |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | AM |  | PM |  |
|  |  | $\begin{gathered} \text { DELAY } \\ \text { (SEC) } \end{gathered}$ | LOS | DELAY <br> (SEC) | LOS |
| Route 209 at Peenpack Trail | NB Left | 7.3 | A | 7.3 | A |
|  | SB Left | 7.3 | A | 7.3 | A |
| (stop sign control) | EB | 11.5 | B | 12.3 | B |
|  | WB | 10.7 | B | 11.0 | B |
| Route 209 at Neversink Dr (stop sign control) | NWB | 10.3 | B | 12.2 | B |
|  | SWB Left | 7.8 | A | 7.9 | A |
| Route 209 at Hanger Dr (stop sign control) | SEB | 10.2 | B | 11.1 | B |
|  | NEB Left | 7.5 | A | 7.8 | A |
| Neversink Dr at Site Dr (stop sign control) | NB Left | 7.4 | A | 7.4 | A |
|  | EB | 9.0 | A | 9.4 | A |

Upon review of the summary table for existing LOS at the key intersection, it is noted that control delays are low - all below 12.5 seconds with all Levels of Service A and B. These results are indicative of very good operating levels with little or no delay at the intersections.

## Future Traffic Conditions

## Background Traffic

As noted above, the project is scheduled to be completed and occupied by the year 2025 about three years from the date of data collection. We would expect that general background traffic growth would occur to account for some minor increases in traffic volumes. This study included a background growth factor, increasing all existing traffic volumes by 4.0 percent. There were no other planned or on-going new developments in the area of the New Century site.

The resulting traffic volumes - projected future traffic without the proposed project - are shown in Figures 3 and 4 in Appendix B for the AM and PM peak hours, respectively. This study refers to this future condition as the "No Build" scenario.

## The Proposed Project

The site is proposed to have the following uses that are expected to generate peak hour traffic:

1. Hotel - 118 rooms
2. Restaurants -500 seats
3. Studio Space (for fabrication and set construction) - 50 employees
4. Studio Space (for film sets) - 50 employees
5. Multi-Purpose Building - for internal meetings and screenings on weekdays, with larger screenings, theatre shows, live events/ festivals and conferences primarily on weekends.

The industry standard trip generation reference (The Trip Generation Manual-11th Edition) from the Institute of Transportation Engineers (ITE) was referenced to estimate traffic for the proposed project. The ITE Land Uses (LU) that best fit the proposed uses are as follows:

1. Hotel-LU310: Hotel - 118 rooms
2. Restaurants - LU931: Fine Dining Restaurant - 500 seats
3. Studio Space (for fabrication and set construction) - LU140: Manufacturing - 50 employees
4. Studio Space (for film sets) - LU150: Warehousing - 50 employees
5. Multi-Purpose Building (weekday use: small screenings and meetings) - LU 445 Movie Theater - one screen

Note: ITE does not have the specific land uses for the proposed "studio" spaces (Items $3 \&$ 4 above). However, ITE does have other uses that can be considered applicable for set construction and other fabrication work associated with filming - ITE's Manufacturing land use. Also, indoor film sets are basically warehousing space that is used for filming. ITE's Manufacturing and Warehousing data were considered applicable for the New Century Film use.

The results of the ITE data and the application of that information as used in this study, with the trip volumes used in the subsequent analyses at the key study intersections, are summarized as follows:

TABLE 3: TRIP GENERATION

| ITE 11th Edition <br> LU310: Hotel |  | Trip Generation |  | 118 rooms |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | Rate (trips/ksf) |  | Volume |  |  |
| Period | Time Frame | Enter | Exit | Enter | Exit |
| AM | Peak Hour of <br> the Adj. Street | 0.25 | 0.20 | 29 | 23 |
| PM | Peak Hour of <br> the Adj. Street | 0.25 | 0.25 | 30 | 29 |


| ITE 11th Edition <br> LU931: Fine Dining Restaurant | Trip Generation |  | 500 seats |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | Rate (trips/ksf) |  | Volume |  |  |
| Period | Time Frame | Enter | Exit | Enter | Exit |
| AM | Peak Hour of <br> the Adj. Street | 0.01 | 0.01 | 5 | 5 |
| PM | Peak Hour of <br> the Adj. Street | 0.19 | 0.09 | 94 | 46 |


| ITE 11th Edition <br> LU140: Manufacturing | Trip Generation |  | 50 employees |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | Rate (trips/ksf) |  | Volume |  |  |
| Period | Time Frame | Enter | Exit | Enter | Exit |
| AM | Peak Hour of <br> the Adj. Street | 0.60 | 0.22 | 30 | 11 |
| PM | Peak Hour of <br> the Adj. Street | 0.32 | 0.56 | 16 | 28 |


| ITE 11th Edition <br> LU150: Warehousing | Trip Generation |  | 50 employees |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | Rate (trips/ksf) |  | Volume |  |  |
| Period | Time Frame | Enter | Exit | Enter | Exit |
| AM | Peak Hour of <br> the Adj. Street | 0.44 | 0.18 | 22 | 9 |
| PM | Peak Hour of <br> the Adj. Street | 0.24 | 0.42 | 12 | 21 |


| ITE 11th Edition <br> LU 445: Movie Theater | Trip Generation |  | 1 screen |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | Rate (trips/ksf) |  | Volume |  |  |
| Period | Time Frame | Enter | Exit | Enter | Exit |
| AM | Peak Hour of <br> the Adj. Street | 0.00 | 0.00 | 0 | 0 |
| PM (Friday) | Peak Hour of <br> the Adj. Street | 14.00 | 11.00 | 14 | 11 |


|  |  | Total Volume |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Period | Time Frame |  | Enter | Exit |  |
| AM | Peak Hour of <br> the Adj. Street |  | 86 | 48 |  |
| PM (Friday) | Peak Hour of <br> the Adj. Street |  |  | 166 | 135 |

Note that the traffic totals shown above do not account for "internal" trips made between uses within the site. For example, guests at the hotel can visit a restaurant during their stay or be involved in filming on a temporary basis. Also, hotel guests, diners and company employees may visit the screening room (movie theater) without leaving the site, thus not accounting for additional external trips. Such internal trips could be as high as $25 \%$ to $30 \%$. To be conservative, this study uses a $20 \%$ reduction factor to account for internal trips. The resulting net new external trips are estimated as follows:

TABLE 4: NET NEW EXTERNAL TRIP GENERATION

|  | Internal Trip <br> Reduction | Volume |  |  |
| :---: | :---: | :---: | :---: | :---: |
|  |  |  | Enter | Exit |
| Period |  | Time Frame |  | 69 | 38 |
| AM | Peak Hour of <br> the Adj. Street | $20 \%$ | 133 | 108 |
| PM (Friday) | Peak Hour of <br> the Adj. Street |  |  |  |

Traffic from the proposed development was distributed to the surrounding street network generally in accordance with the existing travel patterns exhibited in the recent manual counts. The resulting trip volume distributions are shown in Figures 5 and 6 of Appendix B for the AM and PM peak times, respectively.

The traffic generated by the site was then added to the above-described No Build traffic scenario resulting in the Build scenario - the future traffic volumes with both other background growth traffic and traffic from the proposed development. The resulting Build traffic is shown in Figures 7 and 8 in Appendix B for the AM and PM peak hours, respectively. Level of Service (LOS) analyses were run for the No Build and Build traffic scenarios, using the same methodology as used for the existing condition analysis. The results are summarized in the following table:

TABLE 5: LEVEL OF SERVICE SUMMARY - NO BUILD TO BUILD COMPARISON

| INTERSECTION | MVMT. | NO BUILD |  |  |  | BUILD |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | AM |  | PM |  | AM |  | PM |  |
|  |  | $\begin{gathered} \text { DELAY } \\ \text { (SEC) } \end{gathered}$ | LOS | $\begin{gathered} \text { DELAY } \\ \text { (SEC) } \end{gathered}$ | LOS | $\begin{gathered} \text { DELAY } \\ \text { (SEC) } \end{gathered}$ | LOS | $\begin{gathered} \text { DELAY } \\ \text { (SEC) } \end{gathered}$ | LOS |
| Route 209 at Peenpack Trail | NB Left | 7.3 | A | 7.3 | A | 7.3 | A | 7.3 | A |
|  | SB Left | 7.3 | A | 7.3 | A | 7.3 | A | 7.3 | A |
| (stop sign control) | EB | 11.7 | B | 12.5 | B | 11.9 | B | 13.5 | B |
|  | WB | 10.8 | B | 11.2 | B | 11.0 | B | 11.6 | B |
| Route 209 at Neversink Dr (stop sign control) | NWB | 10.4 | B | 12.5 | B | 11.3 | B | 17.0 | C |
|  | SWB Left | 7.8 | A | 7.9 | A | 8.0 | A | 8.2 | A |
| Route 209 at Hanger Dr (stop sign control) | SEB | 10.3 | B | 11.2 | B | 10.5 | B | 12.0 | B |
|  | NEB Left | 7.5 | A | 7.8 | A | 7.6 | A | 7.9 | A |
| Neversink Dr at Site Dr (stop sign control) | NB Left | 7.4 | A | 7.4 | A | 7.5 | A | 7.7 | A |
|  | EB | 9.0 | A | 9.4 | A | 9.7 | A | 11.5 | B |

The Build scenario shows a minor change in Level of Service during the PM peak hour on the site driveway at Neversink Drive. Also, during the PM peak hour, there is one other change in Level of Service: a "B" to "C" on the Neversink Drive approach to Route 209 with a minor delay increase of $41 / 2$ seconds. This increase is not considered significant.

Except as noted at the site entrance and on Neversink Drive during the PM peak hour, delay times increase by no more than one second from no-build to build conditions. Therefore, with such minimal changes in delay time, the impacts at the study intersections would not be noticeable in terms of LOS. This is an indication that traffic related impacts for the proposed development generally will be minimal.

## Multi-Purpose Building (B-2)

As noted above, Building B-2 is proposed to accommodate multiple uses such as internal meeting space and film screenings on weekdays, and screenings, theatre shows, live events/festivals and conferences intended for larger attendance and held primarily on weekends. Those larger events would be accommodated in $\pm 3,500$ seats and would be scheduled in advance and held on Saturdays and Sundays.

While the seating appears to represent B-2 as a significant traffic generator, there are many factors that result in much lower trip generation. For example, the proposal for use as a movie/live theater would still provide a single screen/stage. Statistics for movie theater occupancy has rates averaging between $15 \%$ and $20 \%$. Applying that rate to the 3,500 seats gives an audience of $\pm 700$. That audience would generate just under 300 trips - assuming each vehicle carries between 2 and 3 riders. Moreover, several showings/shows per weekend would spread out the audiences, which ultimately would reach a finite limit. Festivals and conferences, which tend to unfold over the course of many hours throughout the weekend, would further reduce the generated trips during a single hour.

For the reasons stated above, the traffic impact of the proposed Multi-Purpose Building would be limited to weekends and is not expected to negatively impact peak weekend traffic conditions.

## Accident Assessment

The NYSDOT provided accident data for the section of Route 209 from Peenpack Trail to Hangar Drive for the last three years - starting June 1, 2019 and ending May 31, 2022. The DOT Summary is shown in Table 6.

There were totals of 2, 8, 9 and 1 accidents in the four years provided by DOT at the intersection and non-intersection locations in the above-described sections of roadways. During that study period, there were three injury accidents and 16 property-damage-only accidents. One accident resulted in a fatality. There were no accidents that were nonreportable.

Table 6: Accident Severity Summary

| NYSDOT QRA ACCIDENT SEVERITY SUMMARY |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Query Number/Name FMO-22-20592 | Acciden | Range | 6/1/2019 | To | 5/31/2022 |
|  | Attribute Query |  |  |  |  |
| Case Year | Injury | Fatality | Property <br> Damage | Non- <br> Reportable | Total |
| 2019 | 0 | 1 | 1 | 0 | 2 |
| 2020 | 0 | 0 | 8 | 0 | 8 |
| 2021 | 3 | 0 | 6 | 0 | 9 |
| 2022 | 0 | 0 | 1 | 0 | 1 |
| Totals: | 3 | 1 | 16 | 0 |  |
| Grand Total: |  |  |  |  | 20 |

Detailed summaries of all accidents are included in Appendix D. Those summaries include information about each accident, such as location, date and time, severity, type, weather and roadway conditions, and apparent contributing factors. Regarding the accident that resulted in a fatality, the contributing factor was given as "Failure to Yield Right of Way, Turning Improper." These factors are considered driver error.
Data for specific locations were extracted from the detailed summaries and are shown in the following Table:

Table 7: Accident Summary by Location

| ACCIDENT SUMMARY BY LOCATION |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| ON STREET | AT OR NEAR CROSSSTREET | NUMBER OF ACCIDENTS |  |  |  |
|  |  | $\begin{array}{\|c\|} \hline 6 / 30 / 19 \text { to } \\ 12 / 31 / 19 \\ \hline \end{array}$ | 2020 | 2021 | $\begin{array}{\|c\|} \hline 1 / 1 / 22 \text { to } \\ 5 / 31 / 22 \\ \hline \end{array}$ |
| ROUTE 209 | PEENPACK TRAIL | 0 | 1 | 1 | 0 |
|  | CORA ROSE LANE | 0 | 0 | 1 | 0 |
|  | tufano lane | 0 | 1 | 0 | 0 |
|  | NEVERSINK DRIVE | 2 | 4 | 4 | 1 |
|  | PRIVATE DRIVEWAY | 0 | 1 | 1 | 0 |
|  | HANGAR DRIVE | 0 | 0 | 2 | 0 |
| NEVERSINK DRIVE | Route 209 | 0 | 1 | 0 | 0 |

There are two methods of measuring the relative safety history of each study intersection:

1) Frequency $=$ Number of Accidents/Year
2) Rate $=$ Number of Accidents per annual Vehicular Volume

Frequency is simply read from the Summary Table for each location and year. For example, in 2021 there were four (4) accidents at the Route 209/Neversink Drive intersection, described in the detailed summaries as follows:

Table 8: 2021 Accident Details - Route 209 at Neversink Drive

| Case <br> Number | Crash <br> Severity | Case <br> Year | Collision Type | Crash Date | Crash Time | Crash Type |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 38827009 | INJURY | 2021 | REAR END | $4 / 17 / 2021$ | $11: 45$ AM | COLLISION WITH MOTOR <br> VEHICLE | V1:(FOLLOWING TOO CLOSELY,NOT APPLICABLE) / <br> V2:(NOT APPLICABLE,NOT APPLICABLE) |
| 38835503 | PDO | 2021 | OTHER | $4 / 29 / 2021$ | $9: 38$ PM | COLLISION WITH DEER | V1:(ANIMAL'S ACTION,NOTAPPLICABLE) |
| 38998030 | INJURY | 2021 | OTHER | $8 / 8 / 2021$ | $12: 26$ PM | RAN OFF ROAD ONLY | V1:(UNSAFE SPEED,DRIVER INEXPERIENCE) |
| 39163190 | PDO | 2021 | REAR END | $12 / 22 / 2021$ | $12: 45$ PM | COLLSION WITH MOTOR <br> VEHICLE | V1:(NOT APPLICABLE,NOT APPLICABLE) /V2:(CELL <br> PHONE (HAND HELD),FOLLOWING TOO CLOSELY) |

One accident - a "ran off the road" crash - resulted in injury and was attributed to "unsafe speed and driver inexperience." Another accident - a deer strike - resulted in property damage only. The remaining two crashes were rear end type and were attributable to following too closely (tailgating) and distracted driving (cell phone).
In this assessment, contributing factors were primarily driver error related or unknown, as no physical roadway or operational issues were reported. This is an indication of good safety conditions.

Frequencies of five to six or more accidents per year - and a consistent frequency in that range for several consecutive years - are typically indicative of a possible unsafe condition that would bear further study. As shown in Table 7, above, none of the studied intersections meet those thresholds.

Accident rates for State highways are determined by calculating the number of accidents that occurred in the study period per million entering vehicles (MEV) in the section of roadway during that period. The number of accidents is the total number shown for the one-year period for all intersection and non-intersection locations listed in the Table above. The MEV is computed from the recent DOT counts as summarized in Appendix A. The 3-year accident total, the MEV for that period, the resulting rates and the comparable Statewide average rate are summarized in the following Table:
Table 9: Accident Rate Comparison

| ROUTE 209 ACCIDENT RECORD COMPARED TO STATEWIDE AVERAGE |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| FULL CALANDER YEAR | ROADWAY | NUMBER OF ACCIDENTS IN THE STATE RIGHT-OFWAY | MILLION <br> ENTERING VEHICLES (MEV)* | CALCULATED RATE (ACCIDENTS/MEV) | STATEWIDE AVERAGE RATE** (ACCIDENTS/MEV) |
| 2020 |  | 7 | 2.04 | 3.43 |  |
| 2021 |  | 9 | 2.04 | 4.41 |  |

SED ON NYSDOT ESTIMATED AADT VOLUME OF 5,600 ENTERING VEHICLES/DAY X 365 DAYS
ee APPENDIX E for the applicable State DOT rate statistics

Since there were totals of seven and nine accidents in the two full calendar years on Route 209, the calculated rates are 3.43 (2020) and 4.41 (2021). These rates are consistent with the statewide average of 3.73 .

Moreover, note the following:

1. Accident frequencies are low - generally four (4) or fewer accidents per year at all intersections in the study area.
2. Yearly accident rates are consistent with the statewide average.
3. There are no indications that there are unsafe conditions within the study area.

It is concluded that the proposed New Century Film development will not adversely impact the accident history in the study area. The added volume from the site will be distributed in several different directions thereby spreading out the potential increases. No safety issues are expected due to site generated traffic.

## Traffic Impacts During Construction

Impacts due to construction traffic will be temporary in nature, lasting for the duration of the on-going building program at the site. Traffic would consist of occasional heavy trucks delivering building materials to the project site and daily traffic from vehicles belonging to construction workers. Typically, large pieces of construction equipment such as bulldozers and excavators are brought to the site (if needed) at the beginning of the project and kept onsite until no longer needed. Construction may also require the temporary, short-term closure of traffic lanes and flagging to direct traffic during the closure. This will be coordinated with the local Police Department if required. Construction workers' vehicles would be parked onsite.

## Conclusions

This Traffic Impact Study and, in particular, the Level of Service and accident analyses summarized above indicate that, while there will be increases in traffic volumes on the adjacent streets and minor increases in control delay times at key intersections, traffic flows and Levels of Service generally would not be negatively impacted. It is concluded that the proposed project will not adversely impact traffic conditions on the adjacent streets and at intersections in the study area.

## APPENDIX A

DOT TRAFFIC VOLUME DATA 2022 INTERSECTION TURNING MOVEMENT COUNTS

| NYSDOT TRAFFIC VOLUMES |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| Route 209 |  |  |  |  |
| FUNCTIONAL_CLASS FACTOR_GROUP |  | $\begin{aligned} & 14 \\ & 40 \end{aligned}$ |  |  |
| MONTH <br> DAY_OF_FIRST_DATA |  | $\begin{gathered} 10 \\ 1 \\ 2018 \end{gathered}$ |  |  |
| SPECIFIC_RECORDER_PLACEMENT 400' E OF TRI STATES CAMP RD |  |  |  |  |
| SEASONAL_FACTOR AXLE FACTOR |  | $\begin{gathered} 1.034 \\ 1 \end{gathered}$ |  |  |
| Time Period |  | Average Hourly Volume |  |  |
| From | To | Eastbound | Westbound | Total |
| 12:00AM | 1:00 AM | 18 | 11 | 29 |
| 1:00 AM | 2:00 AM | 14 | 10 | 24 |
| 2:00 AM | 3:00 AM | 6 | 8 | 14 |
| 3:00 AM | 4:00 AM | 6 | 11 | 17 |
| 4:00 AM | 5:00 AM | 18 | 16 | 34 |
| 5:00 AM | 6:00 AM | 42 | 73 | 115 |
| 6:00 AM | 7:00 AM | 115 | 128 | 243 |
| 7:00 AM | 8:00 AM | 149 | 212 | 361 |
| 8:00 AM | 9:00 AM | 141 | 181 | 322 |
| 9:00 AM | 10:00 AM | 132 | 192 | 324 |
| 10:00 AM | 11:00 AM | 145 | 163 | 308 |
| 11:00 AM | 12:00 PM | 155 | 171 | 326 |
| 12:00 PM | 1:00 PM | 171 | 174 | 345 |
| 1:00 PM | 2:00 PM | 164 | 190 | 354 |
| 2:00 PM | 3:00 PM | 250 | 190 | 440 |
| 3:00 PM | 4:00 PM | 242 | 254 | 496 |
| 4:00 PM | 5:00 PM | 262 | 245 | 507 |
| 5:00 PM | 6:00 PM | 276 | 206 | 482 |
| 6:00 PM | 7:00 PM | 180 | 163 | 343 |
| 7:00 PM | 8:00 PM | 136 | 132 | 268 |
| 8:00 PM | 9:00 PM | 114 | 72 | 186 |
| 9:00 PM | 10:00 PM | 68 | 46 | 114 |
| 10:00 PM | 11:00 PM | 50 | 41 | 91 |
| 11:00 PM | 12:00 AM | 25 | 34 | 59 |
|  | AADT | 2784 | 2827 | 5611 |



| Project New Century Film |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Intersection |  | Route 209 at Peenpack Trail |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| AM PEAK HOUR |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Day/Date | Thursday | 10/27/2022 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 10 | 11 | 12 |  |  |  |
|  |  | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |  |  |  |
|  | Field \# | 4 | 5 | 6 | 10 | 11 | 12 | 7 | 8 | 9 | 1 | 2 | 3 |  |  |  |
| 7:00 AM | 7:15 AM | 4 | 46 | 1 | 1 | 33 | 1 | 0 | 0 | 1 | 2 | 0 | 14 |  |  |  |
| 7:15 AM | 7:30 AM | 4 | 48 | 2 | 1 | 31 | 1 | 0 | 0 | 1 | 2 | 0 | 11 |  |  |  |
| 7:30 AM | 7:45 AM | 5 | 49 | 1 | 0 | 37 | 2 | 0 | 1 | 1 | 3 | 0 | 10 |  |  |  |
| 7:45 AM | 8:00 AM | 4 | 58 | 1 | 1 | 30 | 1 | 0 | 0 | 2 | 5 | 0 | 14 |  |  |  |
| 8:00 AM | 8:15 AM | 6 | 47 | 1 | 2 | 33 | 3 | 0 | 0 | 0 | 4 | 0 | 10 |  |  |  |
| 8:15 AM | 8:30 AM | 6 | 44 | 2 | 2 | 30 | 1 | 1 | 1 | 1 | 5 | 0 | 10 |  |  |  |
| 8:30 AM | 8:45 AM | 5 | 39 | 1 | 1 | 36 | 4 | 1 | 0 | 2 | 5 | 1 | 8 |  |  |  |
| 8:45 AM | 9:00 AM | 4 | 44 | 1 | 1 | 30 | 1 | 1 | 0 | 1 | 4 | 0 | 7 |  |  |  |
| 9:00 AM | 9:15 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 9:15 AM | 9:30 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 9:30AM | 9:45 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 9:45 AM | 10:00 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 7:00 AM | 7:15 AM | 4 | 46 | 1 | 1 | 33 | 1 | 0 | 0 | 1 | 2 | 0 | 14 | 103 |  | 7:15 AM |
| 7:15AM | 7:30 AM | 4 | 48 | 2 | 1 | 31 | 1 | 0 | 0 | 1 | 2 | 0 | 11 | 101 |  | 7:30 AM |
| 7:30AM | 7:45 AM | 5 | 49 | 1 | 0 | 37 | 2 | 0 | 1 | 1 | 3 | 0 | 10 | 109 |  | 7:45 AM |
| 7:45 AM | 8:00 AM | 4 | 58 | 1 | 1 | 30 | 1 | 0 | 0 | 2 | 5 | 0 | 14 | 116 | 429 | 8:00 AM |
| 8:00 AM | 8:15 AM | 6 | 47 | 1 | 2 | 33 | 3 | 0 | 0 | 0 | 4 | 0 | 10 | 106 | 432 | 8:15 AM |
| 8:15AM | 8:30 AM | 6 | 44 | 2 | 2 | 30 | 1 | 1 | 1 | 1 | 5 | 0 | 10 | 103 | 434 | 8:30 AM |
| 8:30AM | 8:45 AM | 5 | 39 | 1 | 1 | 36 | 4 | 1 | 0 | 2 | 5 | 1 | 8 | 103 | 428 | 8:45 AM |
| 8:45 AM | 9:00 AM | 4 | 44 | 1 | 1 | 30 | 1 | 1 | 0 | 1 | 4 | 0 | 7 | 94 | 406 | 9:00 AM |
| 9:00 AM | 9:15 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 300 | 9:15 AM |
| 9:15 AM | 9:30 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 197 | 9:30 AM |
| 9:30AM | 9:45 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 94 | 9:45 AM |
| 9:45 AM | 10:00 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 10:00 AM |
|  |  | 21 | 198 | 5 | 5 | 130 | 7 | 1 | 2 | 4 | 17 | 0 | 44 | 116 | 434 |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | 7:30AM | 8:30 AM |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | PHF | 0.94 |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  | PEAK HOUR SUMMARY |  |  | EB | L | 21 |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | EB | T | 198 |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | EB | R | 5 |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | WB | L | 5 |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | WB | T | 130 |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | WB | R | 7 |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | NB |  | 1 |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | NB | T | 2 |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | NB | R | 4 |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | SB | L | 17 |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | SB | T | 0 |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | SB | R | 44 |





| Project |  | New Century Film |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Intersection |  | Route 209 at Hangar Drive |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| AM PEAK HOUR |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Day/Date | Thursday | 10/27/2022 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 10 | 11 | 12 |  |  |  |
|  |  | EBL | EB T | EB R | WB L | WBT | WB R | NB L | NBT | NB R | SBL | SB T | SBR |  |  |  |
|  | Field \# | 4 |  |  |  |  | 1 |  |  |  | 2 |  | 3 |  |  |  |
| 7:00 AM | 7:15 AM | 1 |  |  |  |  | 4 |  |  |  | 4 |  | 5 |  |  |  |
| 7:15 AM | 7:30 AM | 2 |  |  |  |  | 1 |  |  |  | 5 |  | 5 |  |  |  |
| 7:30 AM | 7:45 AM | 0 |  |  |  |  | 2 |  |  |  | 5 |  | 7 |  |  |  |
| 7:45 AM | 8:00 AM | 1 |  |  |  |  | 3 |  |  |  | 2 |  | 7 |  |  |  |
| 8:00 AM | 8:15 AM | 2 |  |  |  |  | 3 |  |  |  | 2 |  | 6 |  |  |  |
| 8:15 AM | 8:30 AM | 1 |  |  |  |  | 2 |  |  |  | 8 |  | 3 |  |  |  |
| 8:30 AM | 8:45 AM | 3 |  |  |  |  | 6 |  |  |  | 10 |  | 10 |  |  |  |
| 8:45 AM | 9:00 AM | 1 |  |  |  |  | 4 |  |  |  | 4 |  | 8 |  |  |  |
| 9:00 AM | 9:15 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 9:15 AM | 9:30 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 9:30 AM | 9:45 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 9:45 AM | 10:00 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 7:00 AM | 7:15 AM | 1 | 0 | 0 | 0 | 0 | 4 | 0 | 0 | 0 | 4 | 0 | 5 | 14 |  | 7:15 AM |
| 7:15 AM | 7:30 AM | 2 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 5 | 0 | 5 | 13 |  | 7:30 AM |
| 7:30 AM | 7:45 AM | 0 | 0 | 0 | 0 | 0 | 2 | 0 | 0 | 0 | 5 | 0 | 7 | 14 |  | 7:45 AM |
| 7:45 AM | 8:00 AM | 1 | 0 | 0 | 0 | 0 | 3 | 0 | 0 | 0 | 2 | 0 | 7 | 13 | 54 | 8:00 AM |
| 8:00 AM | 8:15 AM | 2 | 0 | 0 | 0 | 0 | 3 | 0 | 0 | 0 | 2 | 0 | 6 | 13 | 53 | 8:15 AM |
| 8:15 AM | 8:30 AM | 1 | 0 | 0 | 0 | 0 | 2 | 0 | 0 | 0 | 8 | 0 | 3 | 14 | 54 | 8:30 AM |
| 8:30 AM | 8:45 AM | 3 | 0 | 0 | 0 | 0 | 6 | 0 | 0 | 0 | 10 | 0 | 10 | 29 | 69 | 8:45 AM |
| 8:45 AM | 9:00 AM | 1 | 0 | 0 | 0 | 0 | 4 | 0 | 0 | 0 | 4 | 0 | 8 | 17 | 73 | 9:00 AM |
| 9:00 AM | 9:15 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 60 | 9:15 AM |
| 9:15 AM | 9:30 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 46 | 9:30 AM |
| 9:30 AM | 9:45 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 17 | 9:45 AM |
| 9:45 AM | 10:00 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 10:00 AM |
|  |  | 7 | 0 | 0 | 0 | 0 | 15 | 0 | 0 | 0 | 24 | 0 | 27 | 29 | 73 |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | 8:00 AM | 9:00 AM |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | PHF | 0.63 |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  | PEAK HOUR SUMMARY |  |  | EB | L | 7 |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | EB | T | 196 |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | WB | T | 125 |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | WB | R | 15 |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | SB | L | 24 |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | SB | R | 27 |



## APPENDIX B

TRAFFIC VOLUME DIAGRAMS

| FIGURE |  | TITLE |
| :---: | :---: | :---: |
| 1 | AM Peak Hour | 2022 Existing Volumes |
| 2 | PM Peak Hour | 2022 Existing Volumes |
| 3 | AM Peak Hour | 2025 No Build Volumes |
| 4 | PM Peak Hour | 2025 No Build Volumes |
| 5 | AM Peak Hour | Site Generated Traffic Volumes |
| 6 | PM Peak Hour | Site Generated Traffic Volumes |
| 7 | AM Peak Hour | 2025 Build Volumes |
| 8 | PM Peak Hour | 2025 Build Volumes |










APPENDIX C
DETAILED LEVEL OF SERVICE SUMMARIES

| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh | 9.7 |  |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |  |
| Lane Configurations |  | \$ |  |  | 4 |  |  | * |  |  | * |  |  |
| Traffic Vol, veh/h | 21 | 198 | 5 | 5 | 130 | 7 | 1 | 2 | 4 | 17 | 0 | 44 |  |
| Future Vol, veh/h | 21 | 198 | 5 | 5 | 130 | 7 | 1 | 2 | 4 | 17 | 0 | 44 |  |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  |
| Sign Control S | Stop | Stop | Stop | Stop | Stop | Stop | Free | Free | Free | Free | Free | Free |  |
| RT Channelized | - | - | None | - | - | None | - | - | None | - | - | None |  |
| Storage Length | - | - | - | - | - | - | - | - | - | - | - | - |  |
| Veh in Median Storage, \# | \# | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |  |
| Grade, \% | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |  |
| Peak Hour Factor | 94 | 94 | 94 | 94 | 94 | 94 | 94 | 94 | 94 | 94 | 94 | 94 |  |
| Heavy Vehicles, \% | 2 | 10 | 2 | 2 | 10 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |  |
| Mvmt Flow | 22 | 211 | 5 | 5 | 138 | 7 | 1 | 2 | 4 | 18 | 0 | 47 |  |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |


| Major/Minor | Major1 |  | Major2 |  | Minor1 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 0 | 0 | 227 | 0 | 437 | 212 |
| Stage 1 | - | - | - |  | 212 | - |
| Stage 2 | - | - | - | - | 225 | - |
| Critical Hdwy | - | - | 4.15 | - | 6.45 | 6.25 |
| Critical Hdwy Stg 1 | - | - | - | - | 5.45 | - |
| Critical Hdwy Stg 2 | - | - | - | - | 5.45 | - |
| Follow-up Hdwy | - | - | 2.245 |  | 3.545 | 3.345 |
| Pot Cap-1 Maneuver | - | - | 1324 | - | 571 | 821 |
| Stage 1 | - | - | - |  | 816 | - |
| Stage 2 | - | - | - | - | 805 | - |
| Platoon blocked, \% | - | - |  | - |  |  |
| Mov Cap-1 Maneuver | - | - | 1324 | - | 549 | 821 |
| Mov Cap-2 Maneuver | - | - | - | - | 549 | - |
| Stage 1 | - | - | - | - | 816 | - |
| Stage 2 | - | - | - | - | 774 | - |
|  |  |  |  |  |  |  |
| Approach | EB |  | WB |  | NB |  |
| HCM Control Delay, s | 0 |  | 2.1 |  | 10.3 |  |
| HCM LOS |  |  |  |  | B |  |
| HCMLOS |  |  |  |  |  |  |
| Minor Lane/Major Mumt |  | NBLn1 | EBT | EBR | WBL | WBT |
| Capacity (veh/h) |  | 740 | - | - | 1324 | - |
| HCM Lane V/C Ratio |  | 0.082 | - | - | 0.036 | - |
| HCM Control Delay (s) |  | 10.3 | - | - | 7.8 | 0 |
| HCM Lane LOS |  | B | - | - | A | A |
| HCM 95th \%tile Q(veh) |  | 0.3 | - | - | 0.1 | - |




| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 0.5 |  |  |  |  |  |
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
| Lane Configurations | M |  |  | -1 | a |  |
| Traffic Vol, veh/h | 3 | 2 | 3 | 56 | 71 | 4 |
| Future Vol, veh/h | 3 | 2 | 3 | 56 | 71 | 4 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 97 | 97 | 97 | 97 | 97 | 97 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 5 | 5 | 2 |
| Mvmt Flow | 3 | 2 | 3 | 58 | 73 | 4 |



| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh | 10.4 |  |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |  |
| Lane Configurations |  | 4 |  |  | \$ |  |  | * |  |  | * |  |  |
| Traffic Vol, veh/h | 46 | 201 | 0 | 1 | 158 | 12 | 1 | 0 | 2 | 24 | 0 | 46 |  |
| Future Vol, veh/h | 46 | 201 | 0 | 1 | 158 | 12 | 1 | 0 | 2 | 24 | 0 | 46 |  |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  |
| Sign Control | Stop | Stop | Stop | Stop | Stop | Stop | Free | Free | Free | Free | Free | Free |  |
| RT Channelized | - | - | None | - | - | None | - | - | None | - | - | None |  |
| Storage Length | - | - | - | - | - | - | - | - | - | - | - | - |  |
| Veh in Median Storage, \# | \# | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |  |
| Grade, \% | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |  |
| Peak Hour Factor | 94 | 94 | 94 | 94 | 94 | 94 | 94 | 94 | 94 | 94 | 94 | 94 |  |
| Heavy Vehicles, \% | 2 | 7 | 2 | 2 | 7 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |  |
| Mvmt Flow | 49 | 214 | 0 | 1 | 168 | 13 | 1 | 0 | 2 | 26 | 0 | 49 |  |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 3.1 |  |  |  |  |  |
| Movement | EBT | EBR | WBL | WBT | NBL | NBR |
| Lane Configurations | $\uparrow$ |  |  | -1 | M |  |
| Traffic Vol, veh/h | 195 | 36 | 52 | 172 | 47 | 65 |
| Future Vol, veh/h | 195 | 36 | 52 | 172 | 47 | 65 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | 0 | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 91 | 91 | 91 | 91 | 91 | 91 |
| Heavy Vehicles, \% | 7 | 5 | 5 | 7 | 5 | 5 |
| Mvmt Flow | 214 | 40 | 57 | 189 | 52 | 71 |


| Major/Minor M | Major1 |  | Major2 |  | Minor1 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 0 | 0 | 254 | 0 | 537 | 234 |
| Stage 1 | - | - | - | - | 234 | - |
| Stage 2 | - | - |  | - | 303 | - |
| Critical Hdwy | - | - | 4.15 | - | 6.45 | 6.25 |
| Critical Hdwy Stg 1 | - | - | - | - | 5.45 | - |
| Critical Hdwy Stg 2 | - | - | - | - | 5.45 | - |
| Follow-up Hdwy | - | - | 2.245 | - | 3.545 | 3.345 |
| Pot Cap-1 Maneuver | - | - | 1294 | - | 500 | 798 |
| Stage 1 | - | - | - | - | 798 | - |
| Stage 2 | - | - | - | - | 742 | - |
| Platoon blocked, \% | - | - |  | - |  |  |
| Mov Cap-1 Maneuver | - | - | 1294 | - | 476 | 798 |
| Mov Cap-2 Maneuver | - | - | - | - | 476 | - |
| Stage 1 | - | - | - | - | 798 | - |
| Stage 2 | - | - | - | - | 706 | - |
|  |  |  |  |  |  |  |
| Approach | EB |  | WB |  | NB |  |
| HCM Control Delay, s | 0 |  | 1.8 |  | 12.2 |  |
| HCM LOS |  |  |  |  | B |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | NBLn1 | EBT | EBR | WBL WBT |  |
| Capacity (veh/h) |  | 622 | - | - | 1294 | W |
| HCM Lane V/C Ratio |  | 0.198 | - | - | 0.044 | - |
| HCM Control Delay (s) |  | 12.2 | - | - | 7.9 | 0 |
| HCM Lane LOS |  | B | - | - | A | A |
| HCM 95th \%tile Q(veh) |  | 0.7 | - | - | 0.1 | - |




HCM LOS B

| Minor Lane/Major Mvmt | NEL | NET SELn1 | SWT | SWR |
| :--- | ---: | ---: | ---: | ---: |
| Capacity (veh/h) | 1329 | - | 612 | - |







| Major/Minor | Major1 |  | Major2 |  | Minor1 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 0 | 0 | 236 | 0 | 455 | 221 |
| Stage 1 | - | - | - |  | 221 | - |
| Stage 2 | - | - | - | - | 234 | - |
| Critical Hdwy | - | - | 4.15 | - | 6.45 | 6.25 |
| Critical Hdwy Stg 1 | - | - | - | - | 5.45 | - |
| Critical Hdwy Stg 2 | - | - | - | - | 5.45 | - |
| Follow-up Hdwy | - | - | 2.245 |  | 3.545 | 3.345 |
| Pot Cap-1 Maneuver | - | - | 1314 | - | 558 | 811 |
| Stage 1 | - | - | - |  | 809 | - |
| Stage 2 | - | - | - |  | 798 | - |
| Platoon blocked, \% | - | - |  | - |  |  |
| Mov Cap-1 Maneuver | - | - | 1314 | - | 536 | 811 |
| Mov Cap-2 Maneuver | - | - | - | - | 536 | - |
| Stage 1 | - | - | - | - | 809 | - |
| Stage 2 | - | - | - | - | 766 | - |
|  |  |  |  |  |  |  |
| Approach | EB |  | WB |  | NB |  |
| HCM Control Delay, s | 0 |  | 2.1 |  | 10.4 |  |
| HCM LOS |  |  |  |  | B |  |
| HCMLOS |  |  |  |  |  |  |
| Minor Lane/Major Mumt |  | NBLn1 | EBT | EBR | WBL | WBT |
| Capacity (veh/h) |  | 727 | - | - | 1314 | - |
| HCM Lane V/C Ratio |  | 0.088 | - |  | 0.038 | - |
| HCM Control Delay (s) |  | 10.4 | - | - | 7.8 | 0 |
| HCM Lane LOS |  | B | - | - | A | A |
| HCM 95th \%tile Q(veh) |  | 0.3 | - | - | 0.1 | - |




| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 0.5 |  |  |  |  |  |
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
| Lane Configurations | Yr |  |  | -1 | F |  |
| Traffic Vol, veh/h | 3 | 2 | 3 | 58 | 74 | 4 |
| Future Vol, veh/h | 3 | 2 | 3 | 58 | 74 | 4 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 97 | 97 | 97 | 97 | 97 | 97 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 5 | 5 | 2 |
| Mvmt Flow | 3 | 2 | 3 | 60 | 76 | 4 |






| Major/Minor | Major1 |  | Major2 |  | Minor1 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 0 | 0 | 264 | 0 | 559 | 244 |
| Stage 1 | - | - | - |  | 244 | - |
| Stage 2 | - | - | - | - | 315 | - |
| Critical Hdwy | - | - | 4.15 | - | 6.45 | 6.25 |
| Critical Hdwy Stg 1 | - | - | - | - | 5.45 | - |
| Critical Hdwy Stg 2 | - | - | - |  | 5.45 | - |
| Follow-up Hdwy | - | - | 2.245 |  | 3.545 | 3.345 |
| Pot Cap-1 Maneuver | - | - | 1283 | - | 485 | 787 |
| Stage 1 | - | - | - |  | 790 | - |
| Stage 2 | - | - | - |  | 733 | - |
| Platoon blocked, \% | - | - |  | - |  |  |
| Mov Cap-1 Maneuver | - | - | 1283 | - | 460 | 787 |
| Mov Cap-2 Maneuver | - | - | - | - | 460 | - |
| Stage 1 | - | - | - | - | 790 | - |
| Stage 2 | - | - | - | - | 695 | - |
|  |  |  |  |  |  |  |
| Approach | EB |  | WB |  | NB |  |
| HCM Control Delay, s | 0 |  | 1.8 |  | 12.5 |  |
| HCM LOS |  |  |  |  | B |  |
| HCMLOS |  |  |  |  |  |  |
| Minor Lane/Major Mumt |  | NBLn1 | EBT | EBR | WBL | WBT |
| Capacity (veh/h) |  | 606 | - | - | 1283 | - |
| HCM Lane V/C Ratio |  | 0.212 | - | - | 0.046 | - |
| HCM Control Delay (s) |  | 12.5 | - | - | 7.9 | 0 |
| HCM Lane LOS |  | B | - | - | A | A |
| HCM 95th \%tile Q(veh) |  | 0.8 | - | - | 0.1 | - |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 0.7 |  |  |  |  |  |
| Movement | SEL | SER | NEL | NET | SWT | SWR |
| Lane Configurations | Mr |  |  | $\uparrow$ | $\uparrow$ |  |
| Traffic Vol, veh/h | 9 | 7 | 20 | 231 | 206 | 22 |
| Future Vol, veh/h | 9 | 7 | 20 | 231 | 206 | 22 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 7 | 7 | 2 |
| Mvmt Flow | 10 | 8 | 22 | 251 | 224 | 24 |


| Major/Minor | Minor2 |  | Major1 |  | Major2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 531 | 236 | 248 | 0 | - | 0 |
| Stage 1 | 236 | - | - | - | - | - |
| Stage 2 | 295 | - | - | - | - | - |
| Critical Hdwy | 6.42 | 6.22 | 4.12 | - | - | - |
| Critical Hdwy Stg 1 | 5.42 | - | - | - | - | - |
| Critical Hdwy Stg 2 | 5.42 | - | - | - | - | - |
| Follow-up Hdwy | 3.518 | 3.318 | 2.218 | - | - | - |
| Pot Cap-1 Maneuver | 509 | 803 | 1318 | - | - | - |
| Stage 1 | 803 | - | - | - | - | - |
| Stage 2 | 755 | - | - | - | - | - |
| Platoon blocked, \% |  |  |  | - | - | - |
| Mov Cap-1 Maneuver | 499 | 803 | 1318 | - | - | - |
| Mov Cap-2 Maneuver | 499 | - | - | - | - | - |
| Stage 1 | 788 | - | - | - | - | - |
| Stage 2 | 755 | - | - | - | - | - |
|  |  |  |  |  |  |  |
| Approach | SE |  | NE |  | SW |  |
| HCM Control Delay, s | 11.2 |  | 0.6 |  | 0 |  |
| HCM LOS | B |  |  |  |  |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | NEL | NET SELn1 |  | SWT | SWR |
| Capacity (veh/h) |  | 1318 | - | 598 | - | - |
| HCM Lane V/C Ratio |  | 0.016 | - | 0.029 | - | - |
| HCM Control Delay (s) |  | 7.8 | 0 | 11.2 | - | - |
| HCM Lane LOS |  | A | A | B | - | - |
| HCM 95th \%tile Q(veh) |  | 0.1 | - | 0.1 | - | - |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 0.4 |  |  |  |  |  |
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
| Lane Configurations | Yr |  |  | -1 | a |  |
| Traffic Vol, veh/h | 5 | 3 | 1 | 111 | 89 | 2 |
| Future Vol, veh/h | 5 | 3 | 1 | 111 | 89 | 2 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 91 | 91 | 91 | 91 | 91 | 91 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 5 | 5 | 2 |
| Mvmt Flow | 5 | 3 | 1 | 122 | 98 | 2 |





| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 2.8 |  |  |  |  |  |
| Movement | EBT | EBR | WBL | WBT | NBL | NBR |
| Lane Configurations | $\uparrow$ |  |  | $\uparrow$ | M |  |
| Traffic Vol, veh/h | 199 | 58 | 68 | 132 | 29 | 59 |
| Future Vol, veh/h | 199 | 58 | 68 | 132 | 29 | 59 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | 0 | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 97 | 97 | 97 | 97 | 97 | 97 |
| Heavy Vehicles, \% | 7 | 5 | 5 | 7 | 5 | 5 |
| Mvmt Flow | 205 | 60 | 70 | 136 | 30 | 61 |


| Major/Minor M | Major1 |  | Major2 |  | Minor1 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 0 | 0 | 265 | 0 | 511 | 235 |
| Stage 1 | - |  | - | - | 235 | - |
| Stage 2 | - | - | - | - | 276 | - |
| Critical Hdwy | - | - | 4.15 | - | 6.45 | 6.25 |
| Critical Hdwy Stg 1 | - | - | - | - | 5.45 | - |
| Critical Hdwy Stg 2 | - | - | - | - | 5.45 | - |
| Follow-up Hdwy | - | - | 2.245 | - | 3.545 | 3.345 |
| Pot Cap-1 Maneuver | - |  | 1282 | - | 517 | 797 |
| Stage 1 | - | - | - | - | 797 | - |
| Stage 2 | - | - | - | - | 764 | - |
| Platoon blocked, \% | - | - |  | - |  |  |
| Mov Cap-1 Maneuver | - | - | 1282 | - | 486 | 797 |
| Mov Cap-2 Maneuver | - | - | - | - | 486 | - |
| Stage 1 | - | - | - | - | 797 | - |
| Stage 2 | - | - | - | - | 719 | - |
|  |  |  |  |  |  |  |
| Approach | EB |  | WB |  | NB |  |
| HCM Control Delay, s | 0 |  | 2.7 |  | 11.3 |  |
| HCM LOS |  |  |  |  | B |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | NBLn1 | EBT | EBR | R WBL | WBT |
| Capacity (veh/h) |  | 658 | - | - | 1282 | WBT |
| HCM Lane V/C Ratio |  | 0.138 | - |  | 0.055 | - |
| HCM Control Delay (s) |  | 11.3 | - | - | 8 | 0 |
| HCM Lane LOS |  | B | - | - | A | A |
| HCM 95th \%tile Q(veh) |  | 0.5 | - | - | 0.2 | - |








| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 5.3 |  |  |  |  |  |
| Movement | EBT | EBR | WBL | WBT | NBL | NBR |
| Lane Configurations | $\uparrow$ |  |  | -1 | M |  |
| Traffic Vol, veh/h | 203 | 91 | 94 | 179 | 92 | 100 |
| Future Vol, veh/h | 203 | 91 | 94 | 179 | 92 | 100 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | 0 | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 91 | 91 | 91 | 91 | 91 | 91 |
| Heavy Vehicles, \% | 7 | 5 | 5 | 7 | 5 | 5 |
| Mvmt Flow | 223 | 100 | 103 | 197 | 101 | 110 |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 0.6 |  |  |  |  |  |
| Movement | SEL | SER | NEL | NET | SWT | SWR |
| Lane Configurations | M |  |  | -1 | $\uparrow$ |  |
| Traffic Vol, veh/h | 9 | 7 | 20 | 284 | 249 | 22 |
| Future Vol, veh/h | 9 | 7 | 20 | 284 | 249 | 22 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 7 | 7 | 2 |
| Mvmt Flow | 10 | 8 | 22 | 309 | 271 | 24 |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |



APPENDIX D
ACCIDENT DATA

| Case <br> Number | Crash <br> Severity | Case <br> Year | Collision Type | Crash Date | Crash Time | Crash Type | Light <br> Conditions | Road <br> Characteristics | Road Surface Conditions | Weather Conditions | closest Cross Stree | On Street | Apparent Contributing Factor |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 38352159 | FATAL | 2019 | HEAD ON | 7/27/2019 | 11:19 AM | COLLISION WITH MOTOR VEHICLE | DAYLIGHT | $\begin{array}{\|c\|} \hline \text { STRAIGHTAND } \\ \text { LEVEL } \end{array}$ | DRY | CLEAR | NEVERSINK DR | ROUTE 209 | V1:(FAILURE TO YIELD RIGHT OF WAY,TURNING IMPROPER) / V2:(NOT APPLICABLE,NOT APPLICABLE) |
| 38160055 | PDO | 2019 | OTHER | 11/1/2019 | 11:40 AM | COLLISION WITH DEER | DARK-ROAD UNLIGHTED | $\begin{gathered} \text { STRAIGHTAND } \\ \text { LEVEL } \end{gathered}$ | DRY | CLOUDY | NEVERSINK DR | ROUTE 209 | V1:(ANIMAL'S ACTION,NOT APPLICABLE) |
| 38346834 | PDO | 2020 | HEAD ON | 2/24/2020 | 7:45 AM | COLLISION WITH MOTOR VEHICLE | DAYLIGHT | $\begin{array}{\|c\|} \hline \text { STRAIGHTAND } \\ \text { LEVEL } \end{array}$ | DRY | CLEAR | PEENPACK TRL | ROUTE 209 | V1:(TRAFFIC CONTROL DEVICES DISREGARDED,NOT APPLICABLE) / V2:(NOT APPLICABLE,NOT APPLICABLE) |
| 38379442 | PDO | 2020 | OTHER | 3/18/2020 | 7:00 AM | COLLISION WITH SIGN POST | DAYLIGHT | $\begin{array}{\|c\|} \hline \text { STRAIGHTAND } \\ \text { LEVEL } \end{array}$ | DRY | CLEAR | NEVERSINK DR | ROUTE 209 | V1:(UNKNOWN,NOT APPLICABLE) |
| 38401499 | PDO | 2020 | OTHER | 4/28/2020 | 12:45 AM | COLLISION WITH DEER | DARK-ROAD UNLIGHTED | $\begin{array}{\|c\|} \hline \text { STRAIGHT AND } \\ \text { LEVEL } \end{array}$ | DRY | CLEAR | NEVERSINK DR | ROUTE 209 | V1:(ANIMAL'S ACTION,NOT APPLICABLE) |
| 38405337 | PDO | 2020 | OTHER | 4/29/2020 | 7:45 PM | COLLISION WITH GUIDE RAIL | DUSK | CURVEAND LEVEL | DRY | CLOUDY |  |  | V1:(TIRE FAILURE/INADEQUATE,NOT APPLICABLE) |
| 38406255 | PDO | 2020 | OTHER | 5/2/2020 | 11:10 PM | COLLISION WITH DEER | $\begin{gathered} \text { DARK-ROAD } \\ \text { LIGHTED } \\ \hline \end{gathered}$ | $\begin{gathered} \hline \text { CURVE AND } \\ \text { LEVEL } \\ \hline \end{gathered}$ | DRY | CLOUDY | NEVERSINK DR | ROUTE 209 | V1:(ANIMAL'S ACTION,NOT APPLICABLE) |
| 38560939 | PDO | 2020 | OTHER | 9/19/2020 | 7:35 PM | COLLISION WITH DEER | DARK-ROAD UNLIGHTED | $\begin{array}{\|c\|} \hline \text { STRAIGHT AND } \\ \text { LEVEL } \end{array}$ | DRY | CLEAR | DRIVEWAY | ROUTE 209 | V1:(ANIMAL'S ACTION,NOT APPLICABLE) |
| 38741943 | PDO | 2020 | OTHER | 12/10/2020 | 8:05 AM | COLLISION WITH CURBING | DAYLIGHT | CURVEAND LEVEL | SNOW/ICE | CLEAR | ROUTE 209 | NEVERSINK DR | V1:(PAVEMENT SLIPPERY,NOT APPLICABLE) |
| 38689739 | PDO | 2020 | OTHER | 12/29/2020 | 11:00 PM | COLLISION WITH DEER | DARK-ROAD UNLIGHTED | $\begin{gathered} \hline \begin{array}{c} \text { STRAIGHT AND } \\ \text { LEVEL } \end{array} \\ \hline \end{gathered}$ | DRY | CLEAR | NEVERSINK DR | ROUTE 209 | V1:(ANIMAL'S ACTION,NOT APPLICABLE) |
| 38736105 | PDO | 2021 | OTHER | 1/9/2021 | 9:21 AM | COLLISION WITH OTHER FIXED OBJECT | DAYLIGHT | $\begin{array}{\|c\|} \hline \text { STRAIGHTAND } \\ \text { LEVEL } \end{array}$ | DRY | CLEAR | HANGER DR | ROUTE 209 | V1:(GLARE,NOT APPLICABLE) |
| 38827009 | INJURY | 2021 | REAR END | 4/17/2021 | 11:45 AM | COLLISION WITH MOTOR <br> VEHICLE | DAYLIGHT | $\begin{array}{\|c\|} \hline \text { STRAIGHT AND } \\ \text { LEVEL } \end{array}$ | DRY | CLOUDY | NEVERSINK DR | ROUTE 209 | V1:(FOLLOWING TOO CLOSELY,NOT APPLICABLE) / <br> V2:(NOT APPLICABLE,NOT APPLICABLE) |
| 38835503 | PDO | 2021 | OTHER | 4/29/2021 | 9:38 PM | COLLISION WITH DEER | DARK-ROAD UNLIGHTED | $\begin{gathered} \hline \text { STRAIGHTAND } \\ \text { LEVEL } \\ \hline \end{gathered}$ | DRY | CLOUDY | NEVERSINK DR | ROUTE 209 | V1:(ANIMAL'S ACTION,NOT APPLICABLE) |
| 38865188 | INJURY | 2021 | OTHER | 5/20/2021 | 1:20 PM | COLLISION WITHSIGN POST | DAYLIGHT | $\begin{array}{\|c\|} \hline \text { STRAIGHTAND } \\ \text { LEVEL } \end{array}$ | DRY | CLEAR | HANGER DR | ROUTE 209 | V1:(REACTION TO OTHER UNINVOLVED VEHICL,NOT APPLICABLE) |
| 38998030 | INJURY | 2021 | OTHER | 8/8/2021 | 12:26 PM | RAN OFF ROAD ONLY | DAYLIGHT | CURVEAND LEVEL | DRY | CLEAR | NEVERSINK DR | ROUTE 209 | V1:(UNSAFE SPEED,DRIVER INEXPERIENCE) |
| 38985901 | PDO | 2021 | LEFT TURN | 8/14/2021 | 7:25 AM | COLLISION WITH MOTOR VEHICLE | DAYLIGHT | $\begin{array}{\|c\|} \hline \text { STRAIGHT AND } \\ \text { LEVEL } \end{array}$ | WET | RAIN | PEENPACK TRL | ROUTE 209 | V1:(FAILURE TO YIELD RIGHT OF WAY,NOT APPLICABLE)/ V2:(NOT APPLICABLE,NOT APPLICABLE) |
| 39040077 | PDO | 2021 | OTHER | 9/9/2021 | 3:41 AM | COLL. W/EARTH ELE./ROCK CUT/DITCH | DARK-ROAD UNLIGHTED | $\begin{gathered} \hline \text { CURVE AND } \\ \text { LEVEL } \\ \hline \end{gathered}$ | WET | RAIN | DRIVEWAY | ROUTE 209 | V1:(UNSAFE SPEED,FELLASLEEP) |
| 39105779 | PDO | 2021 | OTHER | 11/14/2021 | 9:05 AM | COLLISION WITH DEER | DAYLIGHT | $\begin{array}{\|c\|} \hline \text { STRAIGHT AND } \\ \text { LEVEL } \end{array}$ | DRY | CLEAR | CORA ROSE LN | ROUTE 209 | V1:(ANIMAL'S ACTION,NOT APPLICABLE) |
| 39163190 | PDO | 2021 | REAR END | 12/22/2021 | 12:45 PM | COLLISION WITH MOTOR <br> VEHICLE | DAYLIGHT | $\begin{gathered} \hline \begin{array}{c} \text { STRAIGHTAND } \\ \text { LEVEL } \end{array} \\ \hline \end{gathered}$ | DRY | CLEAR | NEVERSINK DR | ROUTE 209 | V1:(NOT APPLICABLE,NOT APPLICABLE) / V2:(CELL PHONE (HAND HELD),FOLLOWING TOO CLOSELY) |
| 39357018 | PDO | 2022 | OTHER | 5/22/2022 | 6:00 AM | COLL. W/LIGHT SUPPORT/UTILTYY POLE | DAWN | $\begin{array}{\|c\|} \hline \text { STRAIGHTAND } \\ \text { LEVEL } \end{array}$ | DRY | CLEAR | NEVERSINK DR | ROUTE 209 | V1:(ALCOHOL INVOLVEMENT,PASSING OR LANE USAGE IMPROPERLY) |

## APPENDIX E

 DOT STATEWIDE AVERAGE ACCIDENT RATE| URBAN FUNCTION CLASS |  |  |  | MAINLINE \& JUNCTURE ACCIDENTS |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| UNDIVIDED |  |  |  | ALL TYPES ACC/MVM |  |  |
| 2 LANES | 2.38 | 0.44 | 0.34 | 3.73 | 0.68 | 0.44 |
| 3 LANES | 3.34 | 0.6 | 0.28 | 5.31 | 0.95 | 0.38 |
| 4 LANES | 3.57 | 0.69 | 0.19 | 6.41 | 1.22 | 0.31 |
| ALL LANES | 2.64 | 0.49 | 0.32 | 4.27 | 0.79 | 0.43 |
| DIVIDED |  |  |  |  |  |  |
| 2 LANES | 3.45 | 0.64 | 0.2 | 5.56 | 1.02 | 0.32 |
| 4 LANES | 2.99 | 0.56 | 0.18 | 4.63 | 0.87 | 0.25 |
| 6 LANES | 4.14 | 0.77 | 0.15 | 5.53 | 1.01 | 0.18 |
| 7 LANES | 3.51 | 0.6 | 0.06 | 3.82 | 0.69 | 0.07 |
| ALL LANES | 3.36 | 0.63 | 0.17 | 5.02 | 0.94 | 0.26 |



